SEE SHEET S2.1 FOR FOUNDATION SCHEDULE

BOTTOM OF FOOTING ELEVATION = 96'-0" U.O.N.



REGISTRATION SEAL

CONSULTANT

PROJECT TITLE

DEARBORN POOL

TMP Architecture
Bloomfield Hills, Michigan

DRAWING TITLE
FOUNDATION PLAN

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**S1.0** 

ROOF TRUSSES)



**REGISTRATION SEAL** 

CONSULTANT

PROJECT TITLE **DEARBORN POOL** 

TMP Architecture Bloomfield Hills, Michigan

DRAWING TITLE
ROOF FRAMING PLAN

**ISSUE DATES** 10-25-17 BIDS OWNER REVIEW DRAWN **CHECKED** TS

PROJECT NO.

SEE SHEET S2.1 FOR LINTEL SCHEDULE

NOTE #1: BUILD 2'-0" OF MASONRY EACH SIDE OF TRUSS TIGHT TO UNDERSIDE OF 3x TONGUE AND GROOVE ROOF DECKING. SEE DETAIL 11/S3.0

**APPROVED** TS

DRAWING NO.

**S1.1** 

- 2. THE STRUCTURAL DRAWINGS ARE FOR THE PLACEMENT AND SIZE OF STRUCTURAL COMPONENTS ONLY. O.S.H.A., LOCAL GOVERNMENT CODES AND SAFETY CODE REQUIREMENTS SHALL BE ADHERED TO BY THE CONTRACTOR.
- 3. THE STRUCTURE IS DESIGNED TO BE SELF-SUPPORTING AND STABLE AFTER IT IS FULLY COMPLETED. IT IS THE CONTRACTOR'S SOLE RESPONSIBILITY TO DETERMINE ERECTION PROCEDURE AND SEQUENCE, AND TO ENSURE THE SAFETY OF THE STRUCTURE AND ITS COMPONENT PARTS DURING ERECTION. THIS INCLUDES PROVIDING TEMPORARY BRACING, SHORING, GUYS OR TIE- DOWNS. THESE TEMPORARY SUPPORTS WILL REMAIN IN PLACE UNTIL ALL STRUCTURAL COMPONENTS ARE IN PLACE AND COMPLETED.

4. USE OF ENGINEERING DRAWINGS AS ERECTION DRAWINGS BY THE CONTRACTOR IS STRICTLY PROHIBITED. DIMENSIONS SHOWN ON THE STRUCTURAL DRAWINGS ARE FOR REFERENCE ONLY AND SHOULD NOT BE USED FOR BUILDING LAYOUT AND LOCATION. SEE ARCHITECTURAL DRAWINGS AND SITE PLAN FOR THESE PURPOSES.

- 5. CONTRACTOR SHALL CHECK SHOP DRAWINGS PRIOR TO SUBMITTAL AND IS SOLELY RESPONSIBLE FOR ERRORS & OMISSION IN THE PREPARATION OF SHOP DRAWINGS TO CONFORM TO THE DESIGN DRAWINGS. SUBMIT NO MORE THAN ONE REPRODUCIBLE AND TWO PRINTS OF SHOP DRAWINGS FOR ENGINEER REVIEW. TWO COPIES WILL BE RETURNED TO THE ARCHITECT.
- 6. IT IS THE CONTRACTORS RESPONSIBILITY TO VERIFY ALL RELEVANT DIMENSIONS AND ELEVATIONS FOR EQUIPMENT INSTALLATIONS AGAINST PURCHASED MANUFACTURER'S CERTIFIED EQUIPMENT DRAWINGS. DIMENSIONS THAT DEPEND UPON SPECIFIC EQUIPMENT SUCH AS ELEVATOR OPENINGS, MECHANICAL EQUIPMENT SUPPORTS, ETC. SHALL BE COORDINATED BY THE CONTRACTOR PRIOR TO SUBMITTAL TO THE ARCHITECT/ENGINEER. SUCH DIMENSIONS SHALL BE PROVIDED ON THE SHOP DRAWINGS BY THE CONTRACTOR PRIOR TO SUBMITTAL TO THE ARCHITECT/ENGINEER.
- 7. PRE-MANUFACTURED ITEMS SUCH AS CANOPIES, AWNINGS, SUNSHADES, ETC. SHALL BE DESIGNED BY SUPPLIER. SUPPLIER SHAL SUBMIT SIGNED AND SEALED SHOP DRAWINGS AND CALCULATIONS BY A REGISTERED ENGINEER IN THE STATE OF MICHIGAN FOR RECORD TO ARCHITECT. SHOP DRAWINGS SHALL INDICATE ALL DESIGN LOADS AND INCLUDE ALL CONNECTIONS AND MATERIAL NECESSARY FOR INSTALLATION OF PRE-MANUFACTURED ITEMS.

## FOUNDATIONS

- 1. FOOTINGS SHALL BEAR ON FIRM, UNDISTURBED SOIL WITH AN ASSUMED SAFE BEARING CAPACITY OF 2000 P.S.F. IF SOIL OF THIS CAPACITY IS NOT FOUND AT THE ELEVATIONS INDICATED, FOOTINGS SHALL BE ENLARGED OR LOWERED AT THE DIRECTION OF THE ARCHITECT. VERIFY FOUNDATION SOIL BEARING PRESSURE IN FIELD BY SOILS ENGINEER.
- 2. WHERE NEW FOOTINGS ABUT EXISTING FOUNDATIONS, CAREFULLY HAND EXCAVATE AND PLACE BOTTOM OF NEW FOOTING AT THE SAME ELEVATION AS THE EXISTING.
- 3. PROVIDE NECESSARY SHEETING SHORING BRACING, ETC. AS REQUIRED DURING EXCAVATIONS TO PROTECT SIDES OF EXCAVATIONS.
- 4. COMPLY FULLY WITH REQUIREMENTS OF OSHA AND OTHER REGULATORY AGENCIES FOR SAFETY PROVISIONS.

#### CONCRETE

- 1. MINIMUM CONCRETE STRENGTH TO BE 3000 P.S.I. @ 28 DAYS, U.O.N.; SLABS SHALL BE 3500 P.S.I. MIN. U.O.N. EXPOSED CONCRETE SHALL BE 4000 PSI WITH 6% + 1% ENTRAINED AIR U.O.N.
  - A. PROVIDE 3000 P.S.I. 28-DAY COMPRESSIVE STRENGTH: W/C RATIO. 0.58 MAXIMUM (NON-AIR-ENTRAINED), 5.0 BAG CEMENT MIX FOR ALL FOUNDATION WORK UNLESS NOTED OTHERWISE.
  - B. PROVIDE 3500 P.S.I. 28-DAY COMPRESSIVE STRENGTH: W/C RATIO. 0.53 MAXIMUM (NON-AIR-ENTRAINED), 5.5 BAG CEMENT MIX FOR ALL INTERIOR SLABS UNLESS NOTED OTHERWISE.
  - C. PROVIDE 4000 P.S.I. 28-DAY COMPRESSIVE STRENGTH; W/C RATIO, 0.45 MAXIMUM (AIR-ENTRAINED), 6.0 BAG CEMENT MIX FOR ALL EXTERIOR CONCRETE UNLESS NOTED OTHERWISE.
- 2. FLYASH OR GROUND GRANULATED BLAST FURNACE SLAG MAY BE SUBSTITUTED UP TO 25% MAXIMUM OF MIX DESIGN CEMENT CONTENT IN NON-EXPOSED CONCRETE MIXES. DO NOT USE IN EXPOSED MIX DESIGNS.
- 3. ALL CONCRETE WORK AND PLACEMENT SHALL CONFORM TO THE LATEST RECOMMENDATIONS OF A.C.I.
- 4. ALL REINFORCING BARS, DOWELS AND TIES SHALL CONFORM TO A.S.T.M. A615 GRADE 60. REINFORCING STEEL SHALL BE CONTINUOUS AND SHALL HAVE MINIMUM 36 BAR DIAMETER LAP AND BE FABRICATED AND PLACED IN ACCORDANCE WITH A.C.I. - 315 LATEST EDITION.
- 5. REINFORCED CONCRETE WALL FOOTINGS SHALL HAVE CORNER BARS AT ALL INTERSECTIONS OF THE SAME SIZE AND SPACING AS THE MAIN HORIZONTAL REINFORCING.
- 6. ALL SLABS ON GROUND SHALL BE 4" THICK AND HAVE 6" X 6" W1.4 X W1.4 WELDED WIRE FABRIC IN THE TOP 1/3 OF THE SLAB, UNLESS OTHERWISE NOTED.
- 7. FIELD AND SHOP TESTING OF CONCRETE WORK SHALL INCLUDE INSPECTION OF REINFORCING STEEL PLACEMENT, REBARS, NUMBER, LOCATION, AND LAP SPLICE
- 8. PROVIDE DOWELS INTO FOUNDATION TO MATCH SIZE AND SPACING OF VERTICAL REINFORCEMENT AT ALL COLUMNS AND WALLS, UNLESS OTHERWISE NOTED.
- 9. UNLESS OTHERWISE SHOWN, PROVIDE THE FOLLOWING COVER FOR REINFORCING STEEL:

	noing officer		
Α.	UNFORMED SURFACES IN CONTACT WITH EARTH	-3	IN
В.	UNFORMED SURFACES OVER MOISTURE BARRIERS	-2	IN
C.	FORMED SURFACES EXPOSED TO EARTH OR WEATHER		
	OR WATER PROOFING/DAMP PROOFING		
	#6 OR LARGER	-2	IN
	#5 OR SMALLER	-1 1/2	IN
D.	FORMED SURFACES NOT EXPOSED TO EARTH		
	OR WEATHER		
	SLABS AND WALLS	-3/4	IN
	COLUMNS	-1 1/2	IN

# MASONRY

- 1. THE MASONRY PORTIONS OF THIS STRUCTURE ARE DESIGNED ACCORDING TO THE LATEST ALLOWABLE STRESS DESIGN PROVISIONS OF THE MASONRY STANDARDS JOINT COMMITTEE (MSJC) BUILDING CODE REQUIREMENTS FOR MASONRY STRUCTURES (ACI 530/ASCE 602) INCLUDING SECTIONS 2106 AND 2107 OF CHAPTER 21 IN THE MICHIGAN BUILDING CODE. MASONRY COMPONENTS HAVE BEEN DESIGNED ACCORDING TO THE PROVISIONS FOR SEISMIC DESIGN CATEGORY B.
- 2. ALL STRUCTURAL MASONRY IS TO BE CONSTRUCTED IN ACCORDANCE WITH THE LATEST MASONRY STANDARDS JOINT COMMITTEE (MSJC) BUILDING CODE REQUIREMENTS FOR MASONRY STRUCTURES (TMS 402/ACI 530/ASCE 5) AND SPECIFICATIONS FOR MASONRY STRUCTURES (TMS 602/ACI 530.1/ASCE 6) MASONRY SUBMITTALS ARE REQUIRED BY ACI 530.1/ASCE 6/TMS 602. SECTION 1.5 MASONRY TESTING AND INSPECTIONS ARE REQUIRED BY ACI 530.1/ASCE 6/TMS 602 SECTION 1.6, TABLE 5.
- 3. ALL STRUCTURAL MASONRY HAS BEEN ENGINEERED IN ACCORDANCE WITH CHAPTER 2 ALLOWABLE STRENGTH DESIGN. COMPRESSION STRENGTH SHALL BE DETERMINED ACCORDING TO THE UNIT STRENGTH METHOD FOR CONCRETE MASONRY MSJC SECTION 1.4. B.2.b.
- 4. ALL BLOCK SHALL CONFORM TO ASTM C90, TYPE I, WITH A MINIMUM UNIT NET AREA COMPRESSIVE STRENGTH OF 2800 PSI.
- 5. MASONRY COMPRESSIVE STRENGTH f'm = 2000 PSI MINIMUM

BEAMS AND GIRDERS

- 6. MORTAR SHALL BE TYPE "S" (1800 PSI) CONFORMING TO ASTM C-270. USE MORTAR CEMENT WHERE EXTERIOR WALLS ARE UNREINFORCED.
- 7. PROVIDE HORIZONTAL WIRE TYPE REINFORCING WITH 9 GAUGE SIDE AND CROSS MEMBERS IN EVERY SECOND COURSE (16" O.C.), IN ALL MASONRY WALLS. WALLS WITH VERTICAL REINFORCING SHALL ONLY HAVE "LADDER" TYPE REINFORCING.
- 8. ALL REINFORCING BARS, DOWELS AND TIES SHALL CONFORM TO A.S.T.M. A615 GRADE 60. REINFORCING STEEL SHALL BE CONTINUOUS, FABRICATED AND PLACED IN ACCORDANCE WITH A.C.I. - 315 LATEST EDITION AND HAVE THE FOLLOWING MINIMUM LAP LENGTHS:

LOWING WIIN	INION LAI LLIV	arrio.
BAR SIZE	8" CMU	12" CMU
#3	18"	18"
#4	24"	24"
#5	30"	30"
#6	38"	36"
#7		42"
#0		50"

# MASONRY(CONT.)

- 9. ALL MASONRY BEARING STEEL BEAMS AND LINTELS TO BEAR 8" MINIMUM ON 3 COURSES SOLID MASONRY, WITH 2-3/4" DIAMETER BOLTS EACH END, UNLESS OTHERWISE NOTED.
- 10. UNLESS OTHERWISE NOTED WHERE STEEL JOISTS BEAR ON MASONRY, PROVIDE A MINIMUM OF ONE COURSE OF SOLID BLOCK BELOW K-SERIES JOISTS AND A MINIMUM OF TWO COURSES SOLID BELOW LH SERIES JOISTS.
- 11. ALL MASONRY BELOW GRADE SHALL BE GROUTED SOLID.
- 12. MASONRY GROUT SHALL CONFORM TO ASTM C 476, WITH PEA GRAVEL AGGREGATE AND A MINIMUM STRENGTH OF 2000 PSI, BUT NOT LESS THAN SPECIFIED f'm.
- 13. UNLESS OTHERWISE NOTED, AT ALL MASONRY WALLS PROVIDE THE FOLLOWING LINTELS:

# 8" WALLS

(2) L4x3 1/2 x 5/16 LLV FOR OPENINGS UP TO 4'-0" (2) L5x3 1/2 x 5/16 LLV FOR OPENINGS UP TO 5'-4" W8x18 + 3/8" PLATE FOR OPENINGS UP TO 8'-0" W8x28 + 3/8" PLATE FOR OPENINGS UP TO 12'-4"

(3) L4x3- 1/2 x 5/16 LLV FOR OPENINGS UP TO 4'-0"

(3) L5x3-1/2 x 5/16 LLV FOR OPENINGS UP TO 5'-4"

## 12" WALLS:

W8x18 + 3/8" PLATE FOR OPENINGS UP TO 8'-0" W8x28 + 3/8" PLATE FOR OPENINGS UP TO 12'-4"

- 14. ALL DOUBLE ANGLE LINTELS SHALL BE WELDED BACK TO BACK WITH A MINIMUM 2 INCH STITCH WELD EVERY 8 INCHES.
- 15. UNLESS OTHERWISE NOTED, PROVIDE L5 X 3-1/2 X 5/16 L.L.V. LINTEL FOR EACH 4" OF MASONRY FOR SPANS UP TO 5'-0" MAX.
- 16. PROVIDE DOWELS INTO FOUNDATION TO MATCH SIZE AND SPACING OF VERTICAL REINFORCEMENT AT ALL COLUMNS AND WALLS, UNLESS OTHERWISE NOTED.

#### STRUCTURAL STEEL

- 1. STEEL DESIGN, FABRICATION AND ERECTION TO BE IN ACCORDANCE WITH THE LATEST A.I.S.C. MANUAL AND SPECIFICATION FOR STRUCTURAL STEEL FOR BUILDINGS. ALL WIDE FLANGE BEAMS AND COLUMNS SHALL CONFORM TO THE LATEST ASTM. SERIAL DESIGNATION A992, GR50; ALL MISCELLANEOUS STEEL PLATES, BARS, ANGLES, ETC., SHALL CONFORM TO ASTM A36; STEEL TUBING TO BE ASTM A500, GRADE B; STEEL PIPE ASTM. A-53, GRADE B.
- 2. UNLESS OTHERWISE NOTED OR SHOWN, ALL BEAM CONNECTIONS TO HSS 5 X 5 OR SMALLER COLUMN,5"Ø OR SMALLER COLUMN, OR ANY TUBE COLUMN REGARDLESS OF SIZE WITH A WALL THICKNESS LESS THAN 5/16" SHALL BE MADE WITH THRU PLATES WELDED TO BOTH WALLS OF COLUMN.
- 3. ALL WELDED CONNECTIONS SHALL BE IN ACCORDANCE WITH THE LATEST AWS CODE, E70XX ELECTRODES, WITH WELDING PERFORMED BY QUALIFIED WELDERS.
- 4. BOLTED CONNECTIONS SHALL BE MADE WITH A-325 OR A-490 BOLTS. ALL BOLTS ARE TO BE INSTALLED IN ACCORDANCE WITH THE LATEST SPECIFICATIONS FOR "STRUCTURAL JOINTS USING A.S.T.M. A-325 OR A-490 BOLTS." TYPICAL BOLTED CONNECTIONS ARE "BEARING TYPE" UNLESS NOTED OTHERWISE.
- DESIGN CONNECTIONS FOR MINIMUM ONE-HALF THE TOTAL ALLOWABLE UNIFORM LOAD PER A.I.S.C. BEAM LOAD TABLES, UNLESS OTHERWISE NOTED. (MIN. 2 BOLTS EACH CONNECTION).
- 7. SINGLE PLATE SHEAR CONNECTIONS ARE ACCEPTABLE ONLY FOR BEAM TO GIRDER AND SKEWED CONNECTIONS LESS THAN 30 KPS. SHEAR PLATE OR SINGLE SHEAR ANGLES SHALL BE WELDED TO TOP FLANGE OF SUPPORTING GIRDERS.
- 9. THE DESIGN, CONFIGURATION & ERECTION SAFETY OF ALL STRUCTURAL STEEL CONNECTIONS SHALL BE THE RESPONSIBILITY OF THE STRUCTURAL STEEL FABRICATOR. REVIEW AND ACCEPTANCE OF THE SHOP DRAWINGS BY THE ENGINEER SHALL CONSTITUTE APPROVAL OF THE LOAD CARRYING ADEQUACY
- 10. TYPE OF CONSTRUCTION PER ASCE A2.2 IS TYPE 2 "SIMPLE FRAMING" UNLESS NOTED OTHERWISE.
- 11. MASONRY AND BRICK LINTELS SHALL BE GALVANIZED G90 PER ASTM A123.

- 1. WOOD CONSTRUCTION SHALL BE PER AMERICAN INSTITUTE OF TIMBER CONSTRUCTION (AITC) STANDARDS AND SPECIFICATIONS, AND NATIONAL DESIGN SPECIFICATION FOR WOOD CONSTRUCTION (NDS) AS PUBLISHED BY NATIONAL FOREST PRODUCTS ASSOCIATION.
- 2. ALL LUMBER FRAMING MEMBERS ARE TO HAVE THE FOLLOWING MINIMUM BASE DESIGN VALUES IN ACCORDANCE WITH THE LATEST ISSUE OF THE NATIONAL DESIGN SPECIFICATION FOR WOOD CONSTRUCTION (NDS) AS PUBLISHED BY THE NATIONAL FOREST PRODUCTS ASSOCIATION (NFPA):
- Fb = 850 P.S.I.fv = 150 P.S.I.
- E = 1,300,000 P.S.I.NO. 2 OR BETTER
- 3. PLYWOOD TO BE CONTINUOUS OVER TWO (2) OR MORE SPANS AND FACE GRAIN PERPENDICULAR TO SUPPORT.

PLYWOOD OR ORIENTED STRAND BOARD FOR ROOF: 5/8" (32/16) 5 PLY STANDARD GRADE. PROVIDE ROOF CLIPS TYPICAL AT SHEATHING EDGES BETWEEN TRUSSES. STAGGER PLYWOOD JOINTS BETWEEN ROWS OF SHEATHING (OFFSET 4'-0" EACH ROW).

- 4. HANGERS, STRAPS, CLIPS AND HOLDOWNS SHALL BE MANUFACTURED BY THE "SIMPSON MANUFACTURING COMPANY".
- 5. PROVIDE DOUBLE CRIPPLE STUD AT EACH END OF WOOD HEADERS, TYPICAL, UNLESS NOTED OTHERWISE.
- 6. ALL STRUCTURAL GLUED LAMINATED TIMBER TO HAVE THE FOLLOWING MINIMUM DESIGN PROPERTIES BASED ON AITC GRADING STANDARDS:
- Fb = 2400 PSIFv = 265 PSI E = 1,800,000 PSI

-1 1/2 IN.

- 7. GLULAM FRAMING SUPPLIER SHALL SUBMIT DRAWINGS SEALED BY A PROFESSIONAL ENGINEER REGISTERED IN MICHIGAN INDICATING ALL DESIGN LOADS AND MATERIALS INCLUDING VERIFYING ANY MEMBER SIZES SHOWN. DESIGN BY SUPPLIERS ENGINEER SHALL INCLUDE ALL CONNECTIONS AND MISCELLANEOUS MATERIALS AS NECESSARY.
- 8. ALL CONNECTION PLATE MATERIAL SHALL BE A36 MINIMUM.ALL EXTERIOR CONNECTION SHALL BE GALVANIZED.
- 9. CONNECTION PLATE ASSEMBLIES SHALL BE WELDED CONTINUOUS EACH SIDE WITH 1/4" FILLET WELDS
- 10. ALL EXTERIOR GLULAM MEMBERS SHALL BE TREATED WITH PENTACHLOROPHENOL.
- 11. ALL STRUCTURAL LAMINATED VENEER LUMBER TO HAVE THE FOLLOWING MINIMUM DESIGN PROPERTIES BASED ON AITC GRADING STANDARDS:
- Fb = 2800 PSIFv = 285 PSI $Fc_{\perp} = 750 PSI$ E = 2,000,000 PSI
- 12. TIMBER SUPPLIER SHALL BE A MEMBER OF AITC. TIMBER CONNECTIONS TO BE DESIGNED AND DETAILED BY TIMBER SUPPLIER, WITH FABRICATION BY STEEL FABRICATOR.
- 13. UNLESS OTHERWISE NOTED, PROVIDE 2-2X8 HEADER FOR OPENINGS IN STUD WALLS FOR SPANS UP TO 3'-0 MAXIMUM.
- 14. TRUSS MANUFACTURER SHALL PROVIDE SHOP DRAWINGS FOR TRUSSES AND LAYOUT, WITH DESIGN LOADS, SEALED BY A PROFESSIONAL ENGINEER REGISTERED IN THE STATE OF MICHIGAN. TRUSS LAYOUT DRAWINGS SHALL INDICATE ALL REQUIRED BRIDGING FOR TRUSS COMPONENT DESIGN. TRUSS MANUFACTURER SHALL VERIFY WITH ARCHITECT AND MECHANICAL CONTRACTOR SIZE, LOCATION & SUPPORT OF MECHANICAL UNITS. TRUSS FRAMING AND TRUSS TO TRUSS CONNECTIONS ARE TO BE DESIGNED BY TRUSS MANUFACTURER FOR ALL REQUIRED LOADS. SHOP DRAWINGS NOT SIGNED AND SEALED BY A REGISTERED ENGINEER IN THE STATE OF MICHIGAN WILL BE REJECTED. SEE MECHANICAL AND ARCHITECTURAL DRAWINGS FOR MECHANICAL LOADS AT ROOF AND FLOOR TRUSSES. TRUSS SUPPLIER TO DESIGN TRUSSES FOR SUPPORT OF ALL MECH. UNITS, PIPING, FIRE SUPPRESSION LINES, AND ALL UTILITIES. COORDINATE ADDITIONAL LOADING AND PIPE SUPPORT/UTILITY LOCATIONS WITH MECH. CONTRACTOR.
- 15. DESIGN OF THE LUMBER AND CONNECTOR PLATES FOR TRUSSES SHALL BE IN ACCORDANCE WITH LATEST TRUSS PLATE INSTITUTE (TPI) REQUIREMENTS.
- 16. THE TRUSS SUPPLIER SHALL DESIGN AND DETAIL ALL REQUIRED BRIDGING, BRACING AND SUPPLEMENTAL MATERIAL TO PROVIDE A COMPLETE SYSTEM FOR THE LOADS AND PERFORMANCE REQUIREMENTS NOTED.

# WOOD(CONT.)

17. TRUSSES: SHALL BE MANUFACTURED BY AN ACCEPTABLE TRUSS MANUFACTURER, RECOGNIZED BY THE GOVERNING BUILDING CODE. TRUSS MANUFACTURER SHALL SUPPLY ALL HANGERS, PLATES, BLOCKS, CLIPS, BRIDGING AND OTHER ITEMS RELATIVE TO THEIR UNITS. DESIGN CRITERIA ARE AS FOLLOWS:

> ROOF TRUSS: TC LL = 20 PSF DL = 10 PSF BC DL = 15 PSF TL = 45 PSF

CEILING OR CONTINUOUS BRACING AT 10'-0" O.C. PLYWOOD SHEATHING SHALL BE NAILED OR SCREWED TO TRUSS MEMBERS AT 6" O.C. MAXIMUM SPACING.

19. GIRDER TRUSSES SHALL BE DESIGNED TO SUPPORT ALL LOADS FROM THEIR TRIBUTARY AREA.

LATERAL BRACING AT 3 -0" O.C. BOTTOM CHORD MUST BE BRACED WITH A RIGID

- 20. ALL FABRICATION SHOPS SHALL BE APPROVED BY THE BUILDING DEPARTMENT AND ENGINEER PRIOR TO ANY WORK BEING PERFORMED. SUBMIT ALL CERTIFICATIONS AND DOCUMENTATION FOR THEIR REVIEW. WOOD TRUSS DOCUMENTS SUBMITTED BY THE WOOD TRUSS SUPPLIER
- 21. TRUSSES SHALL BE INSTALLED PER THE LATEST TPI BCSI REQUIREMENTS.

IS A "DEFERRED SUBMITTAL" PER SECTION 107.3.4.1 OF THE MBC 2015

18. TRUSS TOP CHORD MUST BE BRACED WITH ROOF SHEATHING OR CONTINUOUS

- 22. POWDER-DRIVEN FASTENERS (P.D.F.) SHALL HAVE MINIMUM SHANK DIAMETER OF 3/16" AND A MINIMUM EMBEDMENT OF 1-1/4". SPACING IS AS NOTED ON THE PLANS. POWDER-DRIVEN FASTENERS SHALL BE AS MANUFACTURED BY "HILTI", "RAMSET", "REDHEAD" OR AN APPROVED EQUAL.
- 23. LET-IN BRACES SHALL NOT BE USED FOR TEMPORARY BRACING ON ANY WALL FRAME. STEEL STRAPS WHICH DO NOT REQUIRE CUTTING OF STUDS ARE ACCEPTABLE.

24. FASTEN WOOD MEMBERS IN ACCORDANCE WITH MBC TABLE 2304.10.1. NAILING SCHEDULE

IF NAILING IS NOT NOTED OR SHOWN ON THE PLANS, SHEARWALL SCHEDULE OR DETAILS, NAILING SCHEDULE IS AS FOLLOWS :

# SCHEDULE IS BASED ON WIRE GAGE NAILS

CONNE	<u>NAILING</u>
1.	JOIST TO SILL OR GIRDER, TOE NAIL
3. 4. 5.	BRIDGING TO JOIST, TOENAIL EACH END
7. 3. 9.	DOUBLE STUDS, FACE NAIL
11. 12. 13. 14. 15.	CEILING JOISTS TO PLATE, TOE NAIL
17. 18. 19.	1"x8" SHEATHING OR LESS TO EACH BEARING, FACE NAIL (2) 80 WIDER THAN 1"x8" SHEATHING TO EACH BEARING, FACE NAIL

- SHEATHING : FLOOR SHEATHING TO FRAMING : 10d NAILS @ 6" O.C. AT EDGES AND @ 12" O.C. AT INTERMEDIATE SUPPORTS. USE RING SHANK NAILS
- WALL SHEATHING TO FRAMING; SEE SHEARWALL SCHEDULE PROVIDE MINIMUM 8d NAILS 6" O.C. AT EDGES AND 12"O.C. AT INTERMEDIATE SUPPORTS
- ROOF SHEATHING TO FRAMING; 8d NAILS @ 6" O.C. AT EDGES AND @ 12" AT INTERMEDIATE SUPPORT U.N.O.
- NOTE: COMMON NAILS ARE REQUIRED FOR SHEAR WALLS INCLUDING PLATE NAILING TIEDOWNS, HANGERS AND LEDGERS
- BOX NAILS REQUIRED 1/3 MORE NAILS THAN LISTED ABOVE. CEMENT COATED SINKERS ARE CONSIDERED BOX NAILS

# SPECIAL INSPECTION

- 1. WORK CONSTRUCTED SHALL BE INSPECTED BY AN INDEPENDENT TESTING AGENCY TO ENSURE COMPLIANCE WITH THE REQUIREMENTS SHOWN ON THE DRAWINGS. INSPECTIONS REQUIRED BY CHAPTER 17 OF THE MICHIGAN BUILDING CODE; LOCAL BUILDING DEPARTMENTS AND THE CONTRACT DOCUMENTS SHALL BE PERFORMED BY AN INDEPENDENT TESTING AGENCY. SITE VISITS BY THE DESIGN ENGINEER DO NOT CONSTITUTE OR REPLACE INSPECTION
- 2. THE FOLLOWING ITEMS SHALL BE INSPECTED IN ACCORDANCE WITH MBC 2015 SEC. 1704 & 1705 BY A CERTIFIED SPECIAL INSPECTOR UNLESS NOTED OTHERWISE IN REMARKS COLUMN. ALL INSPECTION SHALL BE CONTINUOUS UNLESS OTHERWISE NOTED. ALL PRODUCTS WITH ICC APPROVALS SHALL BE INSTALLED PER THE APPROVAL AND PER MANUFACTURER'S RECOMMENDATIONS. FOR MATERIAL TESTING REQUIREMENTS, SEE SPECIFICATIONS AND/OR GENERAL NOTES. TESTING AGENCY SHALL SEND COPIES OF ALL STRUCTURAL TESTING AND
  - INSPECTION OF FABRICATOR'S (SEC. 1704.2.5) \*

INSPECTION REPORTS DIRECTLY TO THE ARCHITECT.

EXCEPTION 1704.2.5.2

- FABRICATION AND IMPLEMENTATION PROCEDURES 1704.2.5.1
- \*SPECIAL INSPECTION IS NOT REQUIRED FOR FABRICATOR SHOP IF CERTIFICATE OF APPROVAL SUBMITTED BY FABRICATOR'S INSPECTION AGENCY PER

#### TABLE 1705.2.2 REQUIRED VERIFICATION AND INSPECTION OF STEEL CONSTRUCTION OTHER THAN STRUCTURAL STEEL

VERIFICATION AND INSPECTION	CONTINUOUS	PERIODIC	NOT APPLICABLE	REFERENCED STANDARD
1. MATERIAL VERIFICATION OF COLD-FORMED STEEL DECK:				
a. IDENTIFICATION MARKINGS TO CONFORM TO ASTM STANDARDS SPECIFIED IN THE APPROVED CONSTRUCTION DOCUMENTS.	-	Х	-	APPLICABLE ASTM MATERIAL STANDARDS
b. MANUFACTURER'S CERTIFIED TEST REPORTS.	-	Х	-	-
2. INSPECTION OF WELDING:	•	•		
a. COLD-FORMED STEEL DECK:				
1) FLOOR AND ROOF DECK WELDS.	-	Х	-	AWS D1.3
b. REINFORCING STEEL:				
<ol> <li>VERIFICATION OF WELDABILITY OF REINFORCING STEEL OTHER THAN ASTM A 706.</li> </ol>	-	Х	-	
2) REINFORCING STEEL RESISTING FLEXURAL AND AXIAL FORCES IN INTERMEDIATE AND SPECIAL MOMENT FRAMES, AND BOUNDARY ELEMENTS OF SPECIAL STRUCTURAL WALLS OF CONCRETE AND SHEAR REINFORCEMENT.	х	-	-	AWS D1.4 ACI 318: SECTION 3.5.2
3) SHEAR REINFORCEMENT.	Х	-	-	
4) OTHER REINFORCING STEEL.	-	Х	-	

TARIE NS 1-1

INSPECTION TASKS PRIOR TO WELDING	QC	QA	A
WELDING PROCEDURE SPECIFICATIONS (WPSs) AVAILABLE	Р	Р	
MANUFACTURER CERTIFICATION FOR WELDING CONSUMABLES AVAILABLE	Р	Р	
MATERIAL IDENTIFICATION (TYPE/GRADE)	0	0	
WELDER IDENTIFICATION SYSTEM <sup>1</sup>	0	0	
FIT-UP OF GROOVE WELDS (INCLUDING JOINT GEOMETRY)  • JOINT PREPARATION  • DIMENSIONS (ALIGNMENT, ROOT OPENING, ROOT FACE, BEVEL)  • CLEANLINESS (CONDITION OF STEEL SURFACES)  • TACKING (TACK WELD QUALITY AND LOCATION)  • BACKING TYPE AND FIT (IF APPLICABLE)	0	0	
CONFIGURATION AND FINISH OF ACCESS HOLES	0	0	
FIT-UP OF FILLET WELDS  • DIMENSIONS (ALIGNMENT, GAPS AT ROOF)  • CLEANLINESS (CONDITION OF STEEL SURFACES)  • TACKING (TACK WELD QUALITY AND LOCATION)	0	0	
CHECK WELDING EQUIPMENT	0	-	

# TABLE N5.4-2

INSPECTION TASKS DURING WELDING	Ī	I	Ī
INSPECTION TASKS DURING TO WELDING	QC	QA.	NOT APPLICABLE
USE OF QUALIFIED WELDERS	0	0	-
CONTROL AND HANDLING OF WELDING CONSUMABLES  • PACKAGING  • EXPOSURE CONTROL	0	0	-
NO WELDING OVER CRACKED TACK WELDS	0	0	-
ENVIRONMENTAL CONDITIONS  • WIND SPEED WITHIN LIMITS  • PRECIPITATION AND TEMPERATURE	0	0	-
WPS FOLLOWED  • SETTINGS ON WELDING EQUIPMENT  • TRAVEL SPEED  • SELECTED WELDING MATERIALS  • SHIELDING GAS TYPE/FLOW RATE  • PREHEAT APPLIED  • INTERPASS TEMPERATURE MAINTAINED (MIN./MAX.)  • PROPER POSITION (F, V, H, OH)	0	0	-
WELDING TECHNIQUES  • INTERPASS AND FINAL CLEANING  • EACH PASS WITHIN PROFILE LIMITATIONS  • EACH PASS MEETS QUALITY REQUIREMENTS	0	0	-

#### TABLE N5.4-3 INSPECTION TASKS AFTER WELDING

INSPECTION TASKS AFTER WELDING	QC	QA.	NOT APPLICABLE
WELDS CLEANED	0	0	-
SIZE, LENGTH AND LOCATION OF WELDS	Р	Р	-
WELDS MEET VISUAL ACCEPTANCE CRITERIA  • CRACK PROHIBITION  • WELD/BASE-METAL FUSION  • CRATER CROSS SECTION  • WELD PROFILES  • WELD SIZE  • UNDERCUT  • POROSITY	P	P	-
ARC STRIKES	P	Р	-
K-AREA <sup>1</sup>	Р	Р	-
BACKING REMOVED AND WELD TABS REMOVED (IF REQUIRED)	Р	Р	-
REPAIR ACTIVITIES	Р	Р	-
DOCUMENT ACCEPTANCE OR REJECTION OF WELDED JOINT OR MEMBER	Р	Р	-
<sup>1</sup> WHEN WELDING OF DOUBLER PLATES, CONTINUITY PLATES OF STIFFENERS HAS BEEN I VISUALLY INSPECT THE WEB K-AREA FOR CRACKS WITHIN 3 IN. (75MM) OF THE WELI		HE K-AREA,	

# TABLE N5.6-1

INSPECTION TASKS PRIOR TO BOLTING	ac	QA	NOT APPLICABLE
MANUFACTURER'S CERTIFICATIONS AVAILABLE FOR FASTENER MATERIALS	0	Р	-
FASTENERS MARKED IN ACCORDANCE WITH ASTM REQUIREMENTS	0	0	-
PROPER FASTENERS SELECTED FOR THE JOINT DETAIL (GRADE, TYPE, BOLT LENGTH IF THREADS ARE TO BE EXCLUDED FROM SHEAR PLANE)	0	0	-
PROPER BOLTING PROCEDURE SELECTED FOR JOINT DETAIL	0	0	-
CONNECTING ELEMENTS, INCLUDING THE APPROPRIATE FAYING SURFACE CONDITION AND HOLE PREPARATION, IF SPECIFIED, MEET APPLICABLE REQUIREMENTS	0	0	-
PRE-INSTALLATION VERIFICATION TESTING BY INSTALLATION PERSONNEL OBSERVED AND DOCUMENTED FOR FASTENER ASSEMBLIES AND METHODS USED	Р	0	-
PROPER STORAGE PROVIDED FOR BOLTS, NUTS, WASHERS AND OTHER FASTNER COMPONENTS	0	0	-

#### TABLE N5.6-2 INSPECTION TASKS DURING BOLTING

INSPECTION TASKS DURING BOLTING	QC	QA	NOT APPLICABLE
FASTENERS ASSEMBLIES, OF SUITABLE CONDITION, PLACED IN ALL HOLES AND WASHERS (IF REQUIRED) ARE POSITIONED AS REQUIRED	0	0	-
JOINT BROUGHT TO THE SNUG-TIGHT CONDITION PRIOR TO THE PRETENSIONING OPERATION	0	0	-
FASTENER COMPONENT NOT TURNED BY THE WRENCH PREVENTED FROM ROTATING	0	0	-
FASTENERS ARE PRETENSIONED IN ACCORDANCE WITH THE RCSC SPECIFICATION, PROGRESSING SYSTEMATICALLY FROM THE MOST RIGID POINT TOWARD THE FREE EDGES	0	0	-

# TARIE N5 6-3

INSPECTION TASKS AFTER BOLTING			
INSPECTION TASKS AFTER BOLTING	QC	QA	NOT APPLICABLE
FASTENER COMPONENT NOT TURNED BY THE WRENCH PREVENTED FROM ROTATING	0	0	-

- O OBSERVE THESE ITEMS ON A RANDOM BASIS. OPERATIONS NEED NOT BE DELAYED PENDING THESE INSPECTIONS.
- P PERFORM THESE TASKS FOR EACH WELDED JOINT OR MEMBER

### TABLE 1705.3 REQUIRED VERIFICATION AND INSPECTION OF CONCRETE CONSTRUCTION

VERIFICATION AND INSPECTION	CONTINUOUS	PERIODIC	NOT APPLICABLE	REFERENCED STANDARD(a)	MBC REFERENCED
INSPECTION OF REINFORCING STEEL INCLUDING     PRESTRESSING TENDONS AND PLACEMENT.	-	Х	-	ACI 318: 3.5, 7.1-7.7	1910.4
2. INSPECTION OF REINFORCING STEEL WELDING IN ACCORDANCE WITH TABLE 1705.2.2, ITEM 2b	-	-	-	AWS D1.4 ACI 318: 3.5.2	-
3. INSPECTION OF ANCHORS CAST IN CONCRETE WHERE ALLOWABLE LOADS HAVE BEEN INCREASED OR WHERE STRENGTH DESIGN IS USED	x	-	-	ACI 318 8.1.3,21.1.8	1908.5 1909.1
4. INSPECTION OF ANCHORS POST-INSTALLED IN HARDENED CONCRETE MEMBERS. (b)	-	х	-	ACI 318 3.8.6,8.1.3,21.1.8	1909.1
5. VERIFYING USE OF REQUIRED DESIGN MIX.	-	х	-	ACI 318: CH.4,5.2-5.4	1904.2, 1910.2,1910.
6. AT THE TIME FRESH CONCRETE IS SAMPLED TO FABRICATE SPECIMENS FOR STRENGTH TESTS, PERFORM SLUMP AND AIR CONTENT TESTS, AND DETERMINE THE TEMPERATURE OF THE CONCRETE.	Х	-	-	ASTM C 172 ASTM C 31 ACI 318: 5.6,5.8	1910.10
7. INSPECTION OF CONCRETE AND SHOTCRETE PLACEMENT FOR PROPER APPLICATION TECHNIQUES.	х	-	-	ACI 318: 5.9,5.10	1910.6, 1910.7,1910.
8. INSPECTION FOR MAINTENANCE OF SPECIFIED CURING TEMPERATURE AND TECHNIQUES.	-	Х	-	ACI 318: 5.11-5.13	1910.9
9. INSPECTION OF PRESTRESSED CONCRETE					
a. APPLICATION OF PRESTRESSING FORCES	Х	-	-	ACI 318: 18.20	
b. GROUTING OF BONDED PRESTRESSING TENDONS IN THE SEISMIC FORCE RESISTING SYSTEM	Х	-	-	ACI 318: 18.18.4	-
10. ERECTION OF PRECAST CONCRETE MEMBERS.	-	х	-	ACI 318: CH.16	-
11. VERIFICATION OF IN-SITU CONCRETE STRENGTH. PRIOR TO STRESSING OF TENDONS IN POSTTENSIONED CONCRETE AND PRIOR TO REMOVAL OF SHORES AND FORMS BEAMS AND STRUCTURAL SLABS.		х	-	ACI 318: 6.2	-
12. INSPECT FORMWORK FOR SHAPE, LOCATION AND DIMENSIONS OF THE CONCRETE MEMBER BEING FORMED.		х	-	ACI 318: 6.1.1	-
FOR SI: 1 INCH = 25.4 MM					

a) WHERE APPLICABLE, SEE ALSO SECTION 1705.11, SPECIAL INSPECTIONS FOR SEISMIC RESISTANCE. (b) SPECIFIC REQUIREMENTS FOR SPECIAL INSPECTION SHALL BE INCLUDED IN THE RESEARCH REPORT FOR THE ANCHOR ISSUED BY AN APPROVED SPECIAL INSPECTION REQUIREMENTS SHALL BE SPECIFIED BY THE REGISTERED DESIGN PROFESSIONAL AND SHALL BE APPROVED BY THE BUILDING OFFICIAL PRIOR TO THE COMMENCEMENT OF THE WORK.



**REGISTRATION SEAL** 

CONSULTANT

PROJECT TITLE **DEARBORN POOL** 

TMP Architecture Bloomfield Hills, Michigan

**ISSUE DATES** 

DRAWING TITLE

**GENERAL NOTES** 

10-25-17

OWNER REVIEW

ISSUED FOR:

**CHECKED** TS **APPROVED** TS

PROJECT NO.

09-27-17

DRAWN

DRAWING NO.

С	OLUMN S	CHEDULE			
			C2	C1	MARK
			HSS6x6	HSS6x6x1/4	SIZE
			12"x3/4"x1'-0"	12"x3/4"x1'-0"	BASE PLATE
			(4)3/4"øx1'-4" HEADED ANCHORS	(4)3/4"øx1'-4" HEADED ANCHORS	ANCHOR BOLTS
			FRAME WOOD MEMBERS TO COLUMN. SEE DETAILS		REMARKS

FOUNDATION SCHEDULE						
				F2	F1,F1A,F1B	MARK
				3'-6"x3'-6"	3'-0"x3'-0"	SIZE
				40"	40"	THICKNESS
				NONE	NONE	REINFORCING EACH WAY-BOTTOM UNLESS OTHERWISE NOTES
					F1A: 76" THICKNESS	REMARKS
					F1B: 64" THICKNESS	

LINTEL SCHEDULE		ULE	8" BEARING EACH END-U.N.O.
MARK	DESCRIPTION		
L1	(2)L4x3 1/2x5/16 LLV		
L2	W8x18 + 1/4" PLATE		
L3	8" WIDEx8" DEEP BOND BEAM WITH 2-#5 BOTTOM		

- LINTEL NOTES: 1. PLATES ON LINTELS EXTEND WIDTH OF MASONRY OPENINGS ONLY. (SEE ARCH. DRAWINGS)
  - HOLD EDGE OF PLATE ON LINTEL BACK FROM EACH FACE OF MASONRY 1/4"
  - WELD 1/2"øx8" HEADED STUDS 32" O.C. TO TOP FLANGE OF ALL WIDE FLANGE LINTELS
  - 4. ALL EXTERIOR LINTELS TO BE GALVANIZED G90 PER ASTM 123

REQUIRED VERIFICATION AND INSPECTION OF MASONRY CONSTRUCTION (LEVEL B QUALITY ASSURANCE)

VERIFICATION OF f'm AND f'ACC IN PRIOR TO CONSTRUCTION, EXCEPT		WITH SPECI					
MIN	IMUM INSPECTI	ON					
		FREQUENCY	(a)		REFERENCE FOR	R CRITERIA	
INSPECTION TASK	CONTINUOUS	PERIODIC	NOT APPLICABLE	IBC SECTION	TMS 402/ACI 530/ASCE 5	TMS 602/AC 530.1/ASCE	
1. VERIFY COMPLIANCE WITH THE APPROVED SUBMITTALS		Х				ART. 1.5	
2. AS MASONRY CONCSTRUCTION BEGINS, VERIFY THAT THE FOLLOWING ARE IN COMPLIANCE:		•					
a. PROPORTIONS OF SITE-PREPARED MORTAR.		Х				ART. 2.1, 2	
b. CONSTRUCTION OF MORTAR JOINTS.		Х				ART. 3.3E	
c. GRADE AND SIZE OF PRESTRESSING TENDONS AND ANCHORAGES.		Х				ART. 2.4B 2.4H	
d. LOCATION OF REINFORCEMENT, CONNECTORS, PRESTRESSING TENDONS AND ANCHORAGES.		Х				ART. 3.4, 3.6A	
e. PRESTRESSING TECHNIQUE.		Х				ART. 3.6E	
f. PROPERTIES OF THIN-BED MORTAR FOR AAC MASONRY	χ <sup>(b)</sup>	X(c)				ART. 2.10	
3. PRIOR TO GROUTING, VERIFY THAT THE FOLLOWING ARE IN COMPLIANCE:							
a. GROUT SPACE		Х				ART. 3.2D 3.2F	
b. GRADE, TYPE AND SIZE OF REINFORCEMENT AND ANCHOR BOLTS, AND PRESTRESSING TENDONS AND ANCHORAGES		х		SEC. 1.16		ART. 2.4, 3	
c. PLACEMENT OF REINFORCEMENT, CONNECTORS, AND PRESTRESSING TENDONS AND ANCHORAGES		Х		SEC. 1.16		ART. 3.2E 3.4, 3.6/	
d. PROPORTIONS OF SITE-PREPARED GROUT AND PRESTRESSING GROUT FOR BONDED TENDONS.							
e. CONSTRUCTION OF MORTAR JOINTS.		Х				ART. 3.3	
4. VERIFY DURING CONSTRUCTION:							
a. SIZE AND LOCATION OF STRUCTURAL ELEMENTS		Х				ART. 3.3F	
b. TYPE, SIZE, AND LOCATION OF ANCHORS, INCLUDING OTHER DETAILS OF ANCHORAGE OF MASONRY TO STRUCTURAL MEMBERS, FRAMES, OR OTHER CONSTRUCTION		х			SEC. 1.16.4.3, 1.17.1		
c. WELDING OF REINFORCEMENT	Х				SEC. 2.1.7.7.2, 3.3.3.4(c), 8.3.3.4(b),		
d. PREPARATION, CONSTRUCTION, AND PROTECTION OF MASONRY DURING COLD WEATHER (TEMPERATURE BELOW 40°F (4.4°C) OR HOT WEATHER (TEMPERATURE ABOVE 90°F (32.2°C)		х				ART. 1.8C 1.8D	
e. APPLICATION AND MEASUREMENT OF PRESTRESSING FORCE	Х					ART. 3.6E	
f. PLACEMENT OF GROUT AND PRESTRESSING GROUT FOR BONDED TENDONS IS IN COMPLIANCE	Х					ART. 3.5, 3	
g. PLACEMENT OF AAC MASONRY UNITS AND CONSTRUCTION OF THIN-BED MORTAR JOINTS	X(p)	X(c)				ART. 3.3 B	
5. OBSERVE PREPARATION OF GROUT SPECIMENS, MORTAR SPECIMENS, AND/OR PRISMS		х				ART. 1.4 B.2. 1.4 B.2.b. 1.4 B.2.c. 1.4 B.3, 1.4	

- OR PERIODICALLY DURING THE LISTED TASK, AS DEFINED IN THE TABLE.
- (b). REQUIRED FOR THE FIRST 5000 SQUARE FEET (465 SQUARE METERS) OF ACC MASONRY. (c). REQUIRED AFTER THE FIRST 5000 SQUARE FEET (465 SQUARE METERS) OF ACC MASONRY.

3. PERFORM CLASSIFICATION AND TESTING OF COMPACTED FILL MATERIALS.

VERIFY USE OF PROPER MATERIALS, DENSITIES AND LIFT THICKNESSES DURING PLACEMENT AND COMPACTION OF COMPACTED FILL.

5. PRIOR TO PLACEMENT OF COMPACTED FILL, OBSERVE SUBGRADE AND VERIFY THAT SITE HAS BEEN PREPARED PROPERLY.

REQUIRED VERIFICATION AND INSPECTION OF SOILS

REQUIRED VERIFICA	ATION AND INSPECTION	OF SOILS
VERIFICATION AND INSPECTION TASK	CONTINUOUS DURING TASK LISTED	PERIODICALLY DURING TASK LISTED
1. VERIFY MATERIALS BELOW SHALLOW FOUNDATIONS ARE ADEQUATE TO ACHIEVE THE DESIGN BEARING CAPACITY.	-	Х
2. VERIFY EXCAVATIONS ARE EXTENDED TO PROPER DEPTH AND HAVE REACHED PROPER MATERIAL.	-	X

NOT APPLICABLE

DECTON	ODITEDIA

- CODE: MBC 2015 THE STRUCTURE IS DESIGNED FOR THE FOLLOWING LIVE LOADS, IN ADDITION TO THE LATERAL LOADS, SUPER-IMPOSED DEAD LOADS, & SELF WEIGHT OF THE STRUCTURE. WHERE APPLICABLE LIVE LOADS ARE REDUCED IN ACCORDANCE WITH THE PROVISIONS OF THE BUILDING CODE.
- A. AMERICAN CONCRETE INSTITUTE BUILDING CODE (ACI-318). B. MANUAL OF STEEL CONSTRUCTION BY AMERICAN INSTITUTE OF STEEL CONSTRUCTION
- C. LATEST MASONRY STANDARDS JOINT COMMITTEE (MSJC) BUILDING CODE REQUIREMENTS FOR MASONRY STRUCTURES (TMS 402/ACI 530/ASCE 5) AND SPECIFICATIONS FOR MASONRY STRUCTURES (TMS 602/ACI 530.1/ASCE 6)
- D. AMERICAN INSTITUTE OF TIMBER CONSTRUCTION (AITC) STANDARDS AND SPECIFICATIONS. E. NATIONAL DESIGN SPECIFICATION FOR WOOD CONSTRUCTION (NDS) AS PUBLISHED BY AMERICAN FOREST AND PAPER ASSOCIATION.

		CODE REFERENCE
BUILDING OCCUPANCY CATEGORY	II	MBC-Table 1604.5 ASCE Table 1.5-1

SNOW LOADS/ROOF LIVE LOADS		
SNOW CRITERIA		CODE REFERENCE
GROUND SNOW LOAD	Pg = 20 PSF	MBC FIG. 1608.2 ASCE Fig. 7-1
FLAT ROOF SNOW LOAD	Pf = 20 PSF (MINIMUM)	ASCE Sec. 7.3
EXPOSURE FACTOR	Ce = 1.0	ASCE Table 7-2
IMPORTANCE FACTOR	I = 1.0	ASCE Table 1.5-2
THERMAL FACTOR	Ct = 1.1	ASCE Table 7-3
ROOF LIVE LOADS	Lr = 20 PSF	ASCE Table 4-1
NOTE: SNOW LOADS ADJACENT VERTICAL PROJECTIONS TO HIGH ROOFS, OR SLOPED ROOFS ARE INCRE		
		·

WIND LOADS		
WIND CRITERIA		CODE REFERENCE
BASIC WIND SPEED (3 SEC. GUST)	V = 115 MPH	ASCE FIG. 26.5-1A, 26.5-1B, 26.5-1C
RISK FACTOR	II	ASCE Table 1.5-1
EXPOSURE CATEGORY	В	ASCE Sec. 26.7.3
INTERNAL PRESSURE COEFFICIENT	± 0.18 (ENCLOSED)	ASCE TABLE 26.11-1
MWFRS ANALYSIS PROCEDURE	DIRECTIONAL PROCEDURE	ASCE CHAP. 27
COMPONENTS AND CLADDING	± 33 PSF MINIMUM AND PER CODE REQUIREMENTS BASED ON ABOVE INFORMATION	ASCE Sec. 30.2.2

SEISMIC LOADS		
SEISMIC CRITERIA		CODE REFERENCE
SEISMIC IMPORTANCE FACTOR	I = 1.0	ASCE Table 1.5-2
-0.2 SEC MAPPED SPECTRAL RESPONSE ACCELERATION (5% OF CRITICAL DAMPING) Ss	Ss = .089	ASCE Sec. 11.4
-1.0 SEC MAPPED SPECTRAL RESPONSE ACCELERATION (5% OF CRITICAL DAMPING) S1	S <sub>1</sub> = .045	ASCE Sec. 11.4
SOIL SITE CLASS	D	ASCE Sec. 11.4.2
SEISMIC DESIGN CATEGORY	В	ASCE Sec. 11.6
SEISMIC FORCE RESISTING SYSTEM	BEARING WALL SYSTEM WITH INTERMEDIATE REINFORCED MASONRY SHEARWALLS	ASCE Table 12.2-1
RESPONSE MODIFICATION FACTOR	R = 3.5	ASCE Table 12.2-1
DEFLECTION AMPLIFICATION FACTOR	Cd = 2.25	ASCE Table 12.2-1
ANALYSIS PROCEDURE	EQUIVALENT LATERAL FORCE	ASCE Sec. 12.8



**REGISTRATION SEAL** 

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PROJECT TITLE **DEARBORN POOL** 

TMP Architecture Bloomfield Hills, Michigan DRAWING TITLE **GENERAL NOTES** 

– (2)3/4"øx1'–4" BOLTS WELDED THROUGH BEARING	8" MINEXTEND  LINTEL FOR PLATES LONGER THAN 7"  AT HEADED STUDS
PLATE INSTALL NUTS FINGER TIGHT AND TACK WELD TO BOLT	TYPICAL
TO PREVENT LOOSENING OR TIGHTENING. COORDINATE END WITH FINGER TIGHTENED	SEE PLAN AND/OR SCHEDULE FOR BEAM OR LINTEL SIZE
NUTS WILL MASONRY CONTROL JOINT	PROVIDE FULL HEIGHT WALL REINFORCING BAR
- 3" LONG×13/16" WIDE HORIZONTAL SLOTTED HOLE IN BEAM FLANGE	
EACH SIDE	(2)3/4"x1'-4" HEADED BOLTS WITH NUTS WELDED THROUGH BEARING PLATE.
—MASONRY WALL	PLATES ON LINTELS STOP AT OPENING WIDTH  PROVIDE 3" LONG×13/16" WIDE HORIZONTAL SLOTTED HOLES IN BEAM FLANGE EACH SIDE
	SEE PLAN AND/OR SCHEDULE FOR BEARING PLATE ON 3 COURSES SOLID  NOTE: FINGER TIGHTEN NUTS ON ONE END. SEE DETAIL 2/S2.1 FOR SLIDE CONNECTION ON SPANS
	3 COURSES SOLID MASONRY (TYP.)  GREATER THAN 8'-0". COORDINATE END WITH FINGER TIGHTENED NUTS WITH MASONRY CONTROL JOINT.
	ADD #5 AT LINTEL  BEARING TYPICAL  TYPICAL LINTEL
DETAIL	1 BEARING ON MASONRY DETAIL
DETAIL	(S2.1) SCALE: $3/4" = 1'-0"$ (LINTEL PARALLEL TO WALL)

TRIM MASONRY AS REQUIRED TO FIT AROUND LINTEL



PROVIDE 1/2"øx8" LONG
HEADED STUDS 32" O.C.
PLACE EACH END OF LINTEL

PROVIDE ADJUSTABLE BRICK TIES 16" O.C. AT HEAD JOINTS

TYPICAL SLIDE CONNECTION LINTEL DETAIL

PROVIDE ON ONE END OF LINTEL FOR LINTEL SPANS GREATER THAN 8'-0"

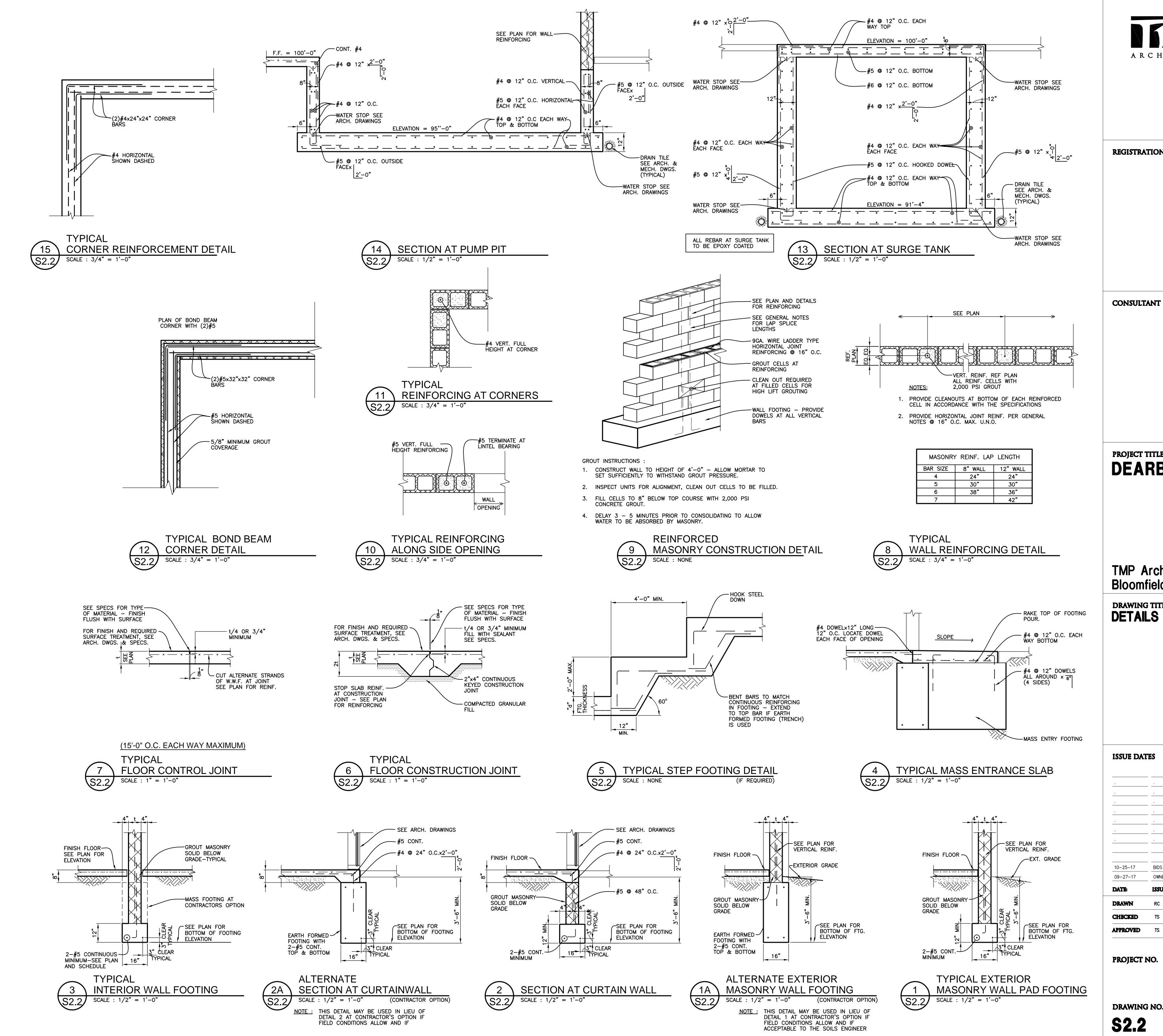
— 3" LONG×13/16" WIDE HORIZONTAL SLOTTED HOLE IN BEAM FLANGE

EACH SIDE

NOTE: PLACE LINTEL BEAMS CENTERED IN CMU WALLS UNLESS NOTED OTHERWISE

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**S2.1** 



ARCHITECTURE

**REGISTRATION SEAL** 

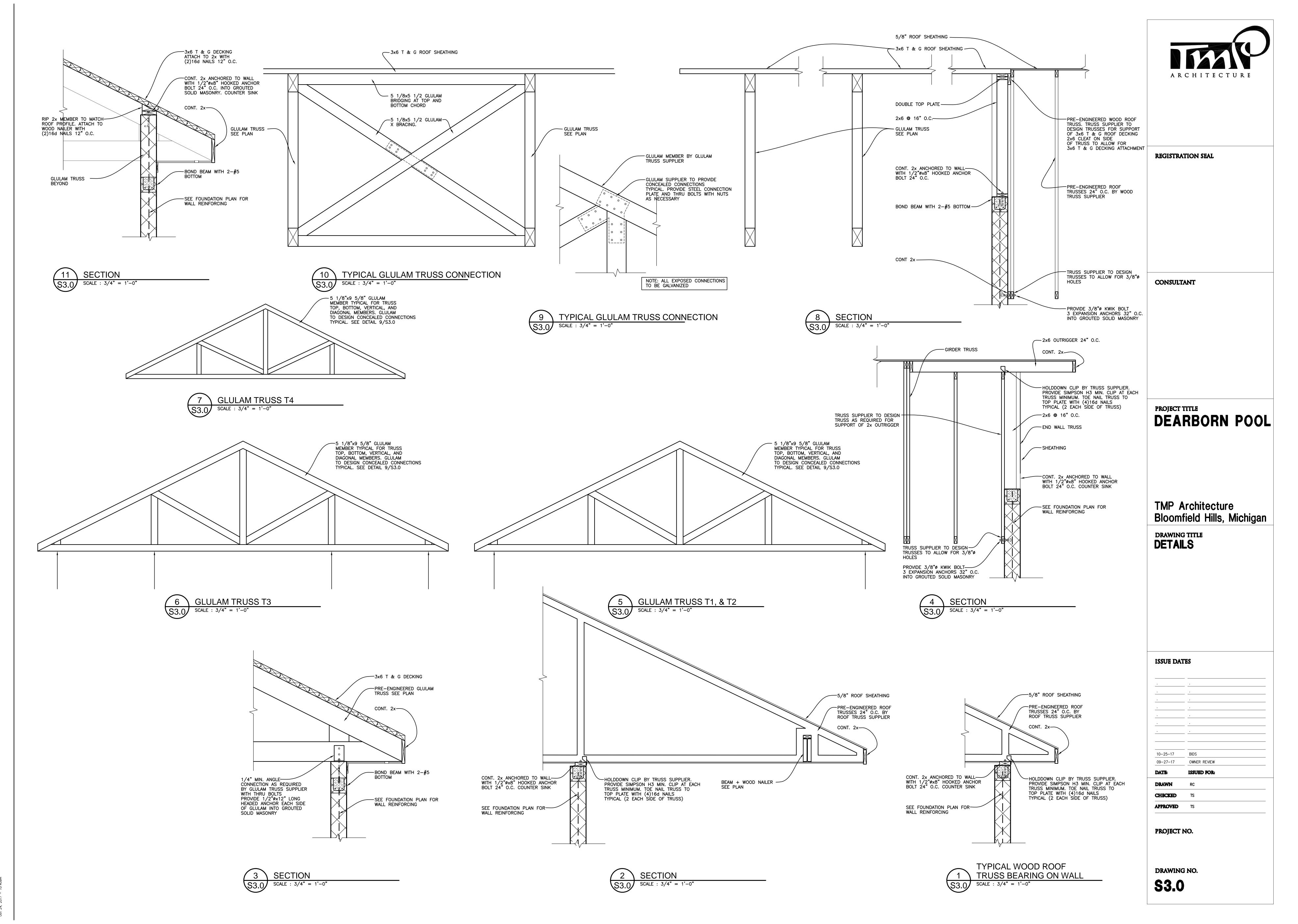
PROJECT TITLE **DEARBORN POOL** 

TMP Architecture Bloomfield Hills, Michigan DRAWING TITLE **DETAILS** 

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**S2.2** 



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